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> # Prof. Dr. Serkan Dağ
# ME 451 Introduction to Composite Structures
> # File 6.2
# Example on computation of extensional ([A]), coupling ([B]),
and bending ([Dm])
# stiffness matrices for a laminate

> restart :
with(LinearAlgebra) :
> # Enter the number of plies
> n := 3 :
> # Define extensional, coupling, and bending stiffness matrices
> A := Matrix(3) :
  B := Matrix(3) :
  Dm := Matrix(3) :
> # Define laminate stiffness matrix
> QL := Matrix(6) :
> # Define ply surface coordinate vector in meters

> h :=  $\begin{bmatrix} -\frac{7.5}{1000} \\ -\frac{2.5}{1000} \\ \frac{2.5}{1000} \\ \frac{7.5}{1000} \end{bmatrix}$  :

> # Define ply angle vector in radians

> theta :=  $\begin{bmatrix} 0 \\ \frac{\text{Pi}}{6} \\ -\frac{\text{Pi}}{4} \end{bmatrix}$  :

> # Define Qbar array
Qbar := Array(1..3, 1..3, 1..n) :
ArrayNumElems(Qbar);

> # Enter properties of the unidirectional lamina
# From Table 2.1 for graphite/epoxy (unit = MPa)
> E1 := 181000 :
  E2 := 10300 :
  nu12 := 0.28 :
  G12 := 7170 :
> # Calculate elements of the compliance matrix for the
unidirectional lamina

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> S11 :=  $\frac{1}{E1}$  :
S12 :=  $-\frac{\nu 12}{E1}$  :
S22 :=  $\frac{1}{E2}$  :
S66 :=  $\frac{1}{G12}$  :
> # Calculate elements of the reduced stiffness matrix for the
unidirectional lamina
> Q11 :=  $\frac{S22}{S11 \cdot S22 - S12^2}$  :
Q22 :=  $\frac{S11}{S11 \cdot S22 - S12^2}$  :
Q12 :=  $-\frac{S12}{S11 \cdot S22 - S12^2}$  :
Q66 :=  $\frac{1}{S66}$  :
> # Calculate elements of transformed reduced stiffness matrix for
each angle lamina
# Unit = MPa
> for i from 1 by 1 to n
while true do
Qbar[1, 1, i] := Q11 · (cos(theta[i, 1]))4 + Q22 · (sin(theta[i, 1]))4 + 2 · (Q12 + 2 · Q66)
· (cos(theta[i, 1]))2 · (sin(theta[i, 1]))2 :
Qbar[1, 2, i] := (Q11 + Q22 - 4 · Q66) · (sin(theta[i, 1]))2 · (cos(theta[i, 1]))2 + Q12
· ((cos(theta[i, 1]))4 + (sin(theta[i, 1]))4) :
Qbar[1, 3, i] := (Q11 - Q12 - 2 · Q66) · (sin(theta[i, 1])) · (cos(theta[i, 1]))3 - (Q22 - Q12
- 2 · Q66) · (sin(theta[i, 1]))3 · cos(theta[i, 1]) :
Qbar[2, 2, i] := Q11 · (sin(theta[i, 1]))4 + Q22 · (cos(theta[i, 1]))4 + 2 · (Q12 + 2 · Q66)
· (cos(theta[i, 1]))2 · (sin(theta[i, 1]))2 :
Qbar[2, 3, i] := (Q11 - Q12 - 2 · Q66) · (cos(theta[i, 1])) · (sin(theta[i, 1]))3 - (Q22 - Q12
- 2 · Q66) · (cos(theta[i, 1]))3 · sin(theta[i, 1]) :
Qbar[3, 3, i] := (Q11 + Q22 - 2 · Q12 - 2 · Q66) · (cos(theta[i, 1]))2 · (sin(theta[i, 1]))2
+ Q66 · ((cos(theta[i, 1]))4 + (sin(theta[i, 1]))4) :
Qbar[2, 1, i] := Qbar[1, 2, i] :
Qbar[3, 1, i] := Qbar[1, 3, i] :
Qbar[3, 2, i] := Qbar[2, 3, i] :
end do
> # Calculate elements of extensional stiffness matrix [A],
coupling stiffness matrix [B], and bending stiffness matrix [Dm]
# Units: [A]--> MPa.m; [B]--> MPa.m2; [Dm]--> MPa.m3
> for i from 1 by 1 to 3
while true do
for j from 1 by 1 to 3
while true do
A[i, j] = 0 :

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B[i,j] := 0 :
Dm[i,j] := 0 :
for k from 1 by 1 to n
while true do
A[i,j] := A[i,j] + Qbar[i,j,k] · (h[k+1,1] - h[k,1]) :
B[i,j] := B[i,j] +  $\frac{1}{2}$  · Qbar[i,j,k] · (h[k+1,1]2 - h[k,1]2) :
Dm[i,j] := Dm[i,j] +  $\frac{1}{3}$  · Qbar[i,j,k] · (h[k+1,1]3 - h[k,1]3) :
end do:
end do:
end do:

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> evalf( A );
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$$\begin{bmatrix} 1739.240863 & 388.3864106 & 56.6337309 \\ 388.3864106 & 453.2535123 & -114.0636096 \\ 56.6337309 & -114.0636096 & 452.4825438 \end{bmatrix}$$

(2)

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> evalf( B );
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$$\begin{bmatrix} -3.128833804 & 0.9855215539 & -1.071656126 \\ 0.9855215539 & 1.157790698 & -1.071656126 \\ -1.071656126 & -1.071656126 & 0.9855215540 \end{bmatrix}$$

(3)

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> evalf( Dm );
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$$\begin{bmatrix} 0.03343203414 & 0.006460977237 & -0.005240293688 \\ 0.006460977237 & 0.009319771318 & -0.005595913147 \\ -0.005240293688 & -0.005595913147 & 0.007662779735 \end{bmatrix}$$

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> # Form laminate stiffness matrix QL by converting stress unit to Pa
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> for i from 1 by 1 to 3
while true do
for j from 1 by 1 to 3
while true do
QL[i,j] := A[i,j] · 106 :
QL[i,j+3] := B[i,j] · 106 :
QL[i+3,j] := B[i,j] · 106 :
QL[i+3,j+3] := Dm[i,j] · 106 :
end do:
end do:

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> # Form loading vector N
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> N :=  $\begin{bmatrix} 1000 \\ 1000 \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix}$  :
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> # Find midplane strains and curvatures
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> Res := evalf( LinearSolve( QL, N ) );
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$$Res := \begin{bmatrix} 3.122948352 \cdot 10^{-7} \\ 0.000003491704604 \\ -7.598445480 \cdot 10^{-7} \\ 0.00002971437363 \\ -0.0003285023896 \\ 0.0004101466831 \end{bmatrix} \quad (5)$$

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> # Calculate global strains
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> epsx := Res[1, 1] + z·Res[4, 1];
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epsy := Res[2, 1] + z·Res[5, 1];
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gammaxy := Res[3, 1] + z·Res[6, 1];
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$$\begin{aligned} epsx &:= 0.00002971437363 z + 3.122948352 \cdot 10^{-7} \\ epsy &:= -0.0003285023896 z + 0.000003491704604 \\ gammaxy &:= 0.0004101466831 z - 7.598445480 \cdot 10^{-7} \end{aligned} \quad (6)$$

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> # Calculate global strains at z = - 2.5 mm
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> subs( z = - 2.5 / 1000, epsx );
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subs( z = - 2.5 / 1000, epsy );
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```
subs( z = - 2.5 / 1000, gammaxy );
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$$\begin{aligned} &2.380089011 \cdot 10^{-7} \\ &0.000004312960578 \\ &-0.000001785211256 \end{aligned} \quad (7)$$

```
> # Calculate global stresses (on 30-degree ply) (in Pa)
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> sigmax := evalf( ( Qbar[1, 1, 2]·epsx + Qbar[1, 2, 2]·epsy + Qbar[1, 3, 2]·gammaxy ) · 106 );
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sigmay := evalf( ( Qbar[2, 1, 2]·epsx + Qbar[2, 2, 2]·epsy + Qbar[2, 3, 2]·gammaxy ) · 106 );
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tauxy := evalf( ( Qbar[3, 1, 2]·epsx + Qbar[3, 2, 2]·epsy + Qbar[3, 3, 2]·gammaxy ) · 106 );
```

$$\begin{aligned} sigmax &:= 1.481317925 \cdot 10^7 z + 1.063300340 \cdot 10^5 \\ sigmay &:= 1.421474740 \cdot 10^6 z + 77467.82379 \end{aligned} \quad (8)$$

$$\tau_{xy} := 1.008968412 \cdot 10^7 z + 59031.78949 \quad (8)$$

> # Calculate global stresses at  $z = -2.5$  mm (on 30-degree ply) (in Pa)

>  $\text{sigx} := \text{subs}\left(z = -\frac{2.5}{1000}, \text{sigmax}\right);$   
 $\text{sigy} := \text{subs}\left(z = -\frac{2.5}{1000}, \text{sigmay}\right);$   
 $\text{txy} := \text{subs}\left(z = -\frac{2.5}{1000}, \text{tauxy}\right);$

$$\text{sigx} := 69297.08588$$

$$\text{sigy} := 73914.13694$$

$$\text{txy} := 33807.57919 \quad (9)$$

> # Compute local stresses at  $z = -2.5$  (on 30-degree ply) (in Pa)

>  $c := \cos\left(\frac{\text{Pi}}{6}\right);$   
 $s := \sin\left(\frac{\text{Pi}}{6}\right);$

>  $\text{sigma1} := \text{evalf}\left(c^2 \cdot \text{sigx} + s^2 \cdot \text{sigy} + 2 \cdot s \cdot c \cdot \text{txy}\right);$   
 $\text{sigma2} := \text{evalf}\left(s^2 \cdot \text{sigx} + c^2 \cdot \text{sigy} - 2 \cdot s \cdot c \cdot \text{txy}\right);$   
 $\text{tau12} := \text{evalf}\left(-s \cdot c \cdot \text{sigx} + s \cdot c \cdot \text{sigy} + (c^2 - s^2) \cdot \text{txy}\right);$

$$\sigma_1 := 99729.57108$$

$$\sigma_2 := 43481.65174$$

$$\tau_{12} := 18903.03136 \quad (10)$$

> # Compute local strains at  $z = -2.5$  mm

>  $\text{eps1} := \frac{1}{E1 \cdot 10^6} \cdot \text{sigma1} - \frac{\nu_{12}}{E1 \cdot 10^6} \cdot \text{sigma2};$   
 $\text{eps2} := -\frac{\nu_{12}}{E1 \cdot 10^6} \cdot \text{sigma1} + \frac{1}{E2 \cdot 10^6} \cdot \text{sigma2};$   
 $\text{gamma12} := \frac{\text{tau12}}{G12 \cdot 10^6};$

$$\text{eps1} := 4.837276718 \cdot 10^{-7}$$

$$\text{eps2} := 0.000004067241796$$

$$\gamma_{12} := 0.000002636406047 \quad (11)$$

> # Load  $N_x$  acting on ply 1

>  $\text{sxx1} := (Q_{bar}[1, 1, 1] \cdot \text{epsx} + Q_{bar}[1, 2, 1] \cdot \text{epsy} + Q_{bar}[1, 3, 1] \cdot \text{gamma}_{xy}) \cdot 10^6;$   
 $N_{x1} := \text{int}(\text{sxx1}, z = h[1, 1] .. h[2, 1]);$

$$N_{x1} := 223.2004826 \quad (12)$$

> # Load  $N_x$  acting on ply 2

>  $\text{sxx2} := (Q_{bar}[1, 1, 2] \cdot \text{epsx} + Q_{bar}[1, 2, 2] \cdot \text{epsy} + Q_{bar}[1, 3, 2] \cdot \text{gamma}_{xy}) \cdot 10^6;$   
 $N_{x2} := \text{int}(\text{sxx2}, z = h[2, 1] .. h[3, 1]);$

$$N_{x2} := 531.6501700 \quad (13)$$

> # Load  $N_x$  acting on ply 3

>  $\text{sxx3} := (Q_{bar}[1, 1, 3] \cdot \text{epsx} + Q_{bar}[1, 2, 3] \cdot \text{epsy} + Q_{bar}[1, 3, 3] \cdot \text{gamma}_{xy}) \cdot 10^6;$

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> Nx3 := int(sxx3, z = h[3, 1]..h[4, 1]);  
Nx3 := 245.1493450  
=>
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**(14)**