

HOUSE PROJECT IN UMITKOY

ARCH 332 STRUCTURAL DESIGN IN ARCHITECTURE

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2013-14, Spring



GROUP 27

MELİH AKCA

EZGİ ATÇAKAN

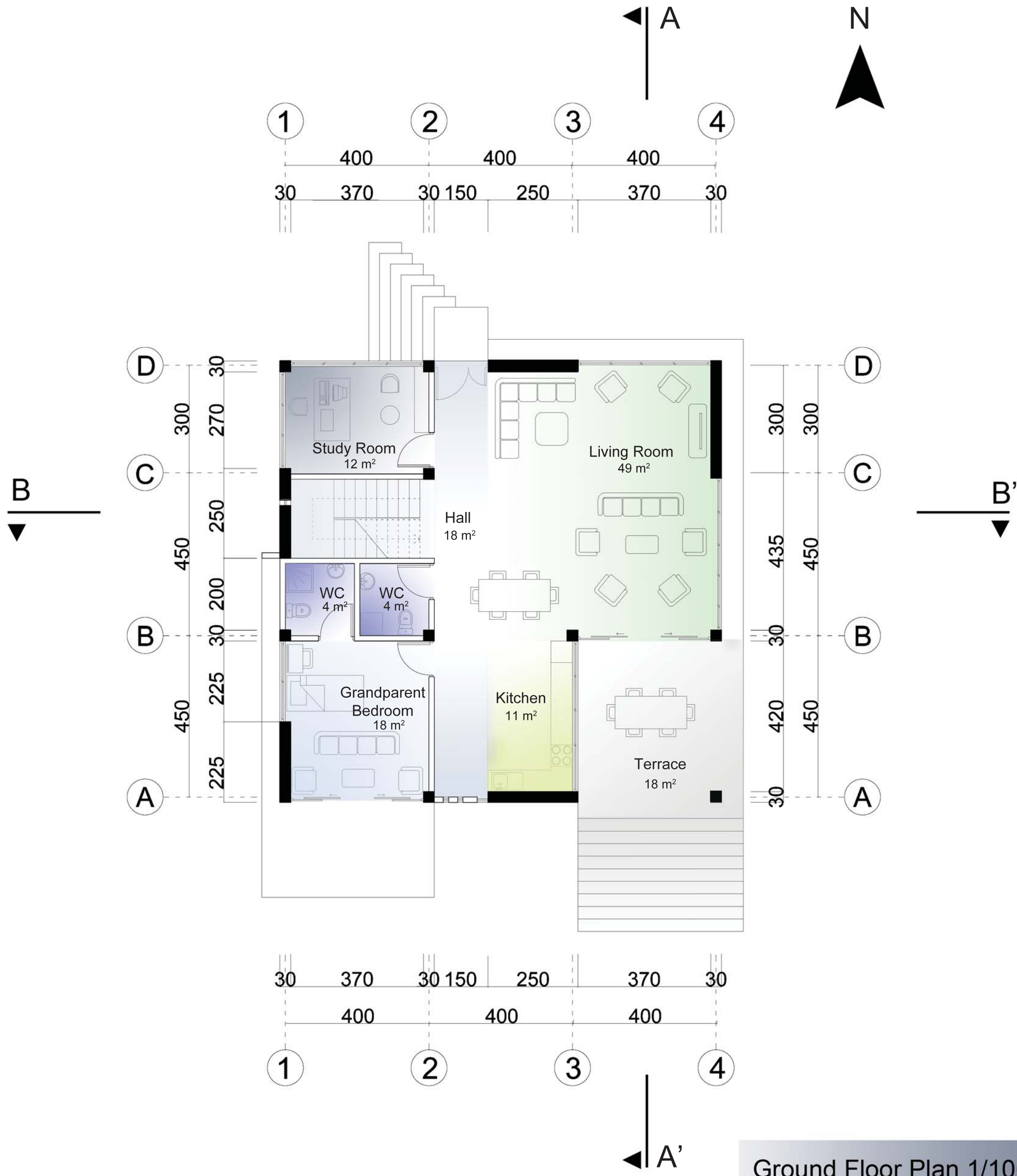
JÜLİDE ARZU ULUÇAY



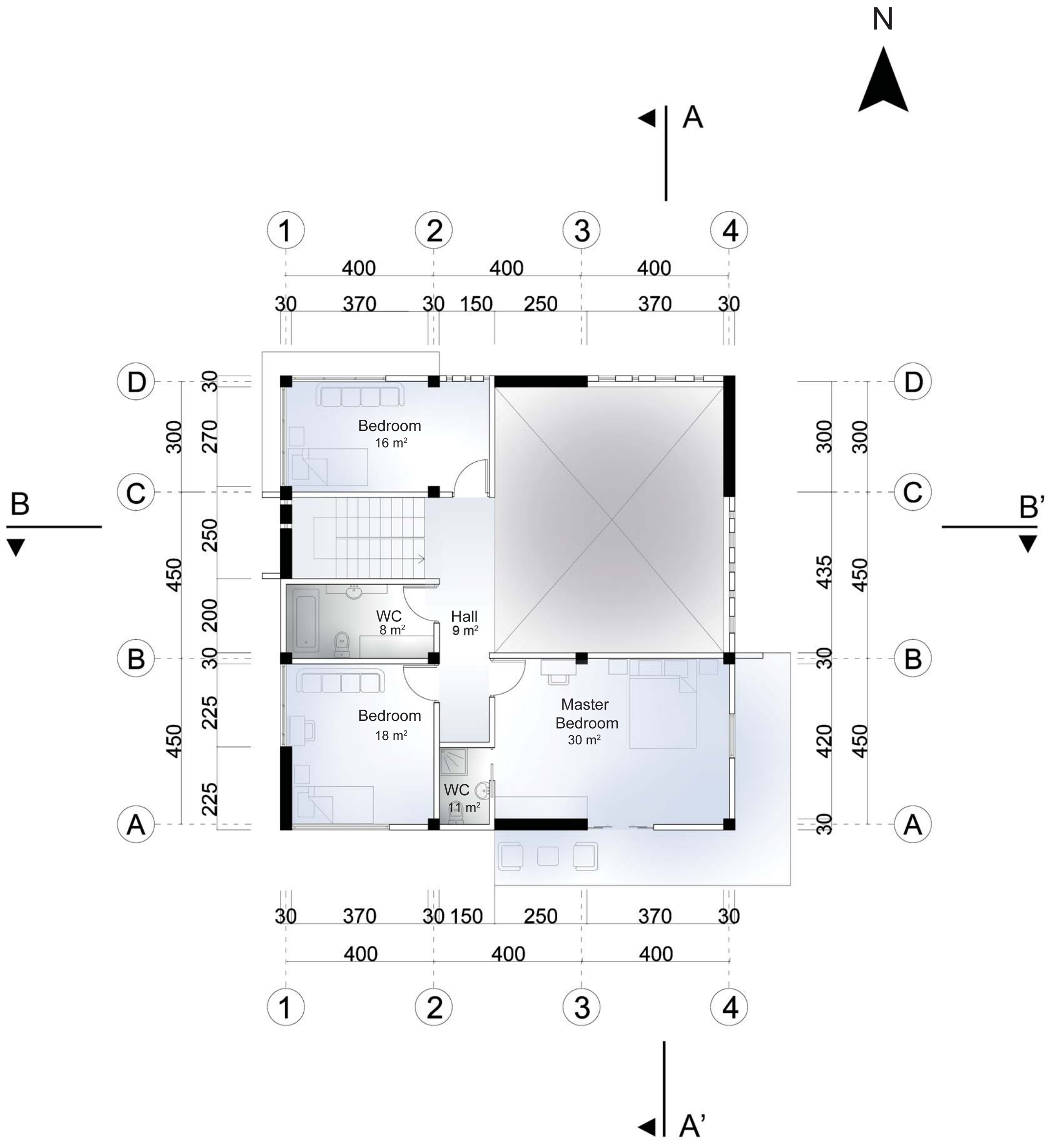
Our aim is to create a living space which connects different area at different floor. In addition, we aimed to associate house to back garden with open air terrace which is at the south part of house.

At facade, we try to emphasize shear wall in terms of color, height and form. We use horizontal and vertical continuous elements as eaves and balconies.

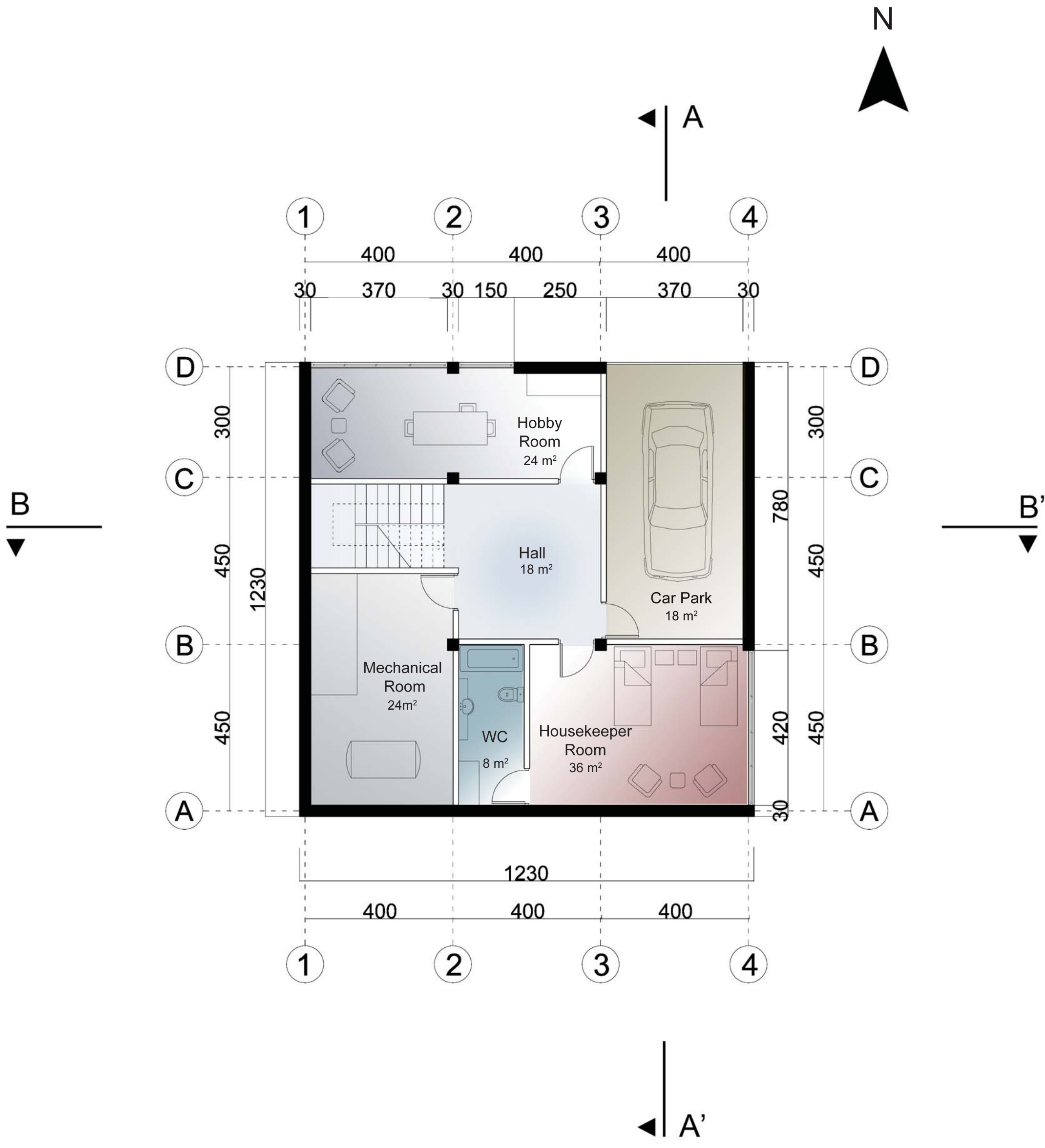




Ground Floor Plan 1/100



First Floor Plan 1/100



Basement Floor Plan 1/100

PERSPECTIVE SECTIONS



SECTION A-A'



SECTION B-B'



Section A-A' 1/100



Section B-B' 1/100

BACK GARDEN



**Open Air
Terrace
connect House
and Back
Garden**



FACADES



North Facade



East Facade



South Facade



West Facade

VIEWS FROM INSIDE

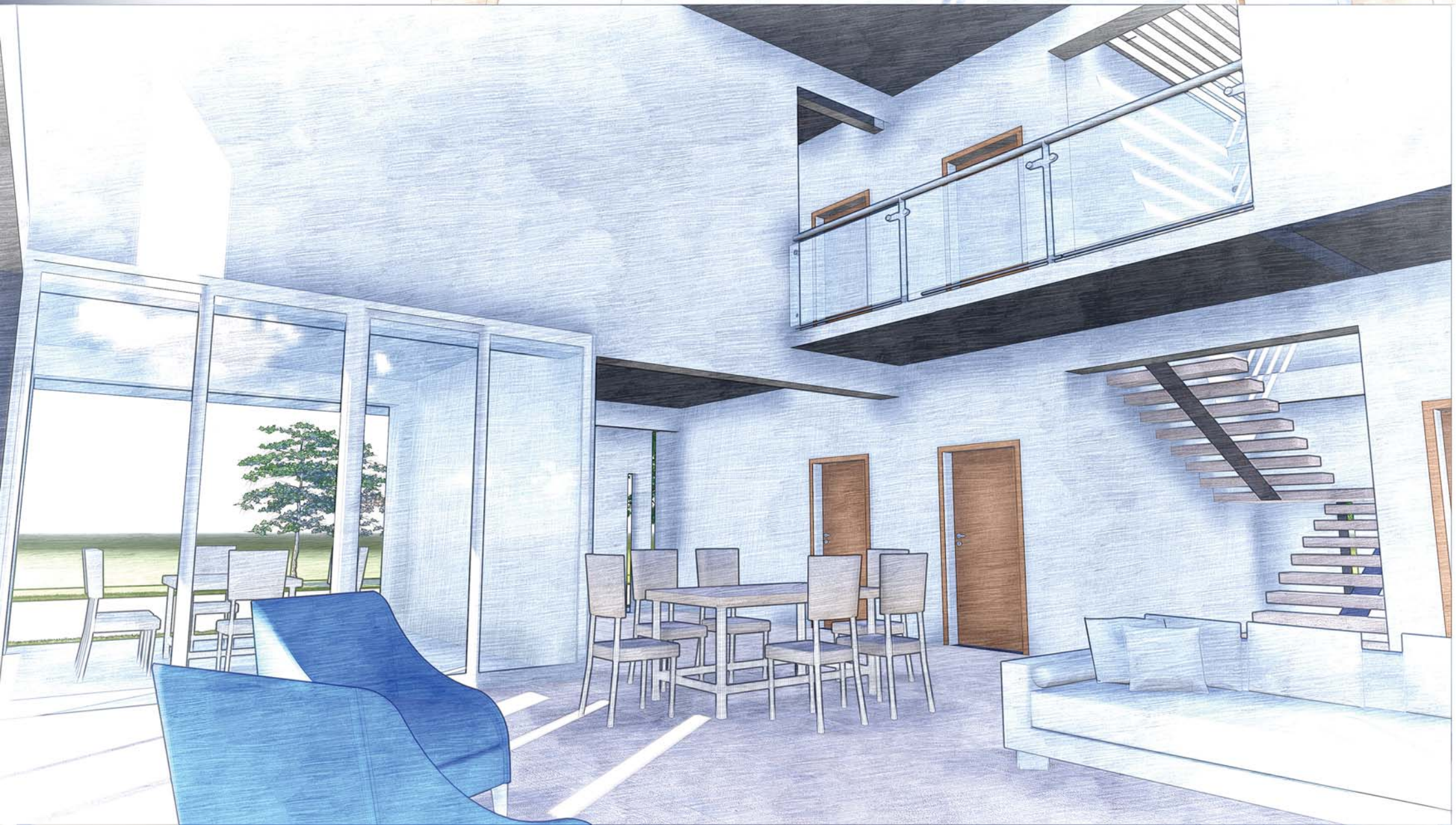


**Living Area
and Gallery Space
with Vertical
Opening**

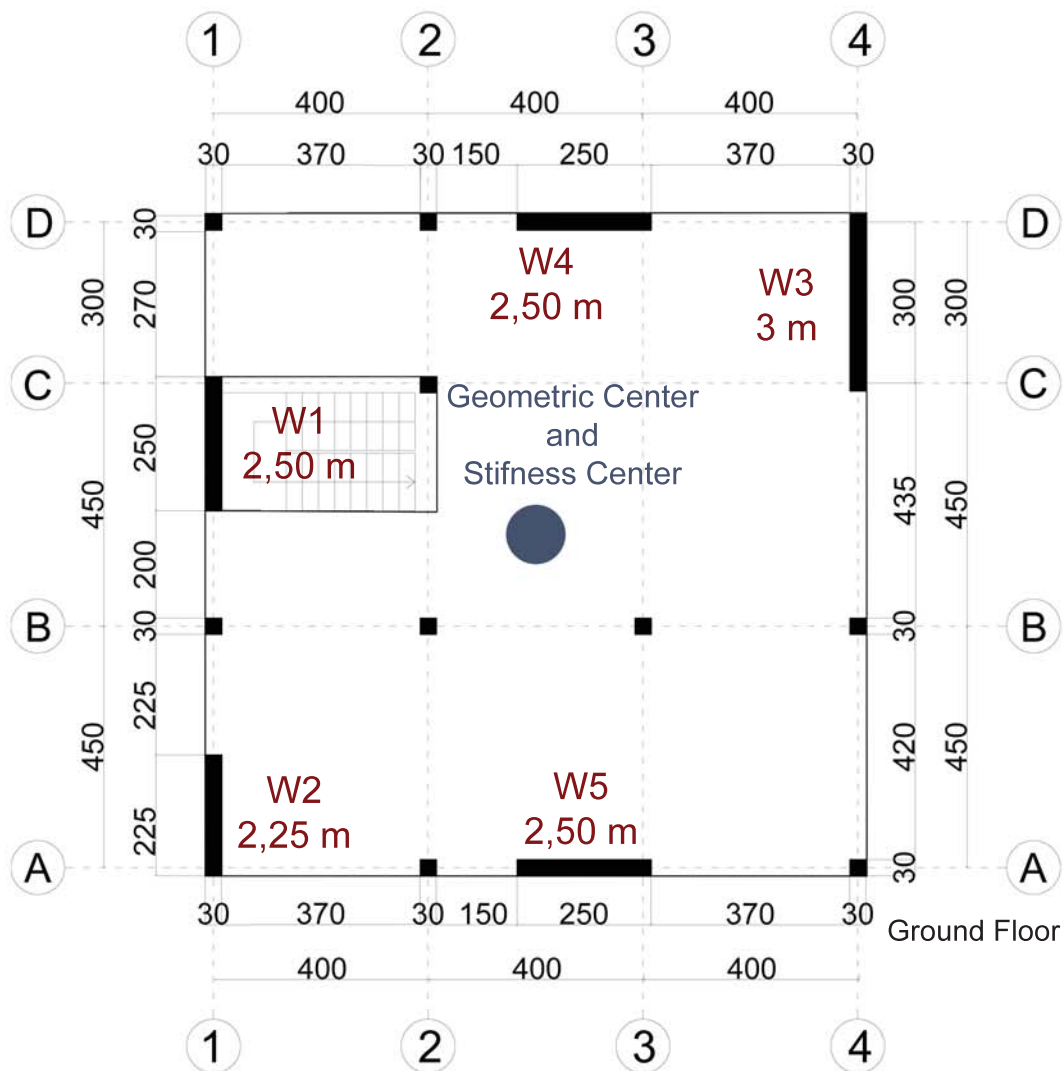
**Sun shading
devices above
Staircase**



**Living Area with
Open Air Terrace
and Galery**



SELECTION OF STRUCTURAL SYSTEM



GEOMETRIC CENTER

Geometric center is at the center of the building because its symmetric

STIFNESS CENTER

$$x_{sc} = \frac{I_1 l_1 + I_2 l_2 + I_3 l_3}{I_1 + I_2 + I_3}$$

$$x_{sc} = \frac{(1/12 \times 0,3 \times (2,5)^3 \times 0) + (1/12 \times 0,3 \times (2,25)^3 \times 0) + (1/12 \times 0,3 \times (3)^3 \times 12)}{(1/12 \times 0,3 \times (2,5)^3) + (1/12 \times 0,3 \times (2,25)^3) + (1/12 \times 0,3 \times (3)^3)}$$

$$= 5,998 \text{ m}$$

$$x_{sc} = 6 \text{ m}$$

$$y_{sc} = \frac{I_4 l_4 + I_5 l_5}{I_4 + I_5}$$

$$y_{sc} = \frac{(1/12 \times 0,3 \times (2,5)^3 \times 12) + (1/12 \times 0,3 \times (2,25)^3 \times 0)}{(1/12 \times 0,3 \times (2,5)^3) \times 2}$$

$$y_{sc} = 6 \text{ m}$$

Stiffness center is 6;6 so stiffness center and geometric center are coincided.

SHEAR WALL PERCENTAGE

Ground floor area: 144 m^2

Area of shear wall on x direction:

$$0,3 \times 2,50 \times 2 = 1,5 \text{ m}^2$$

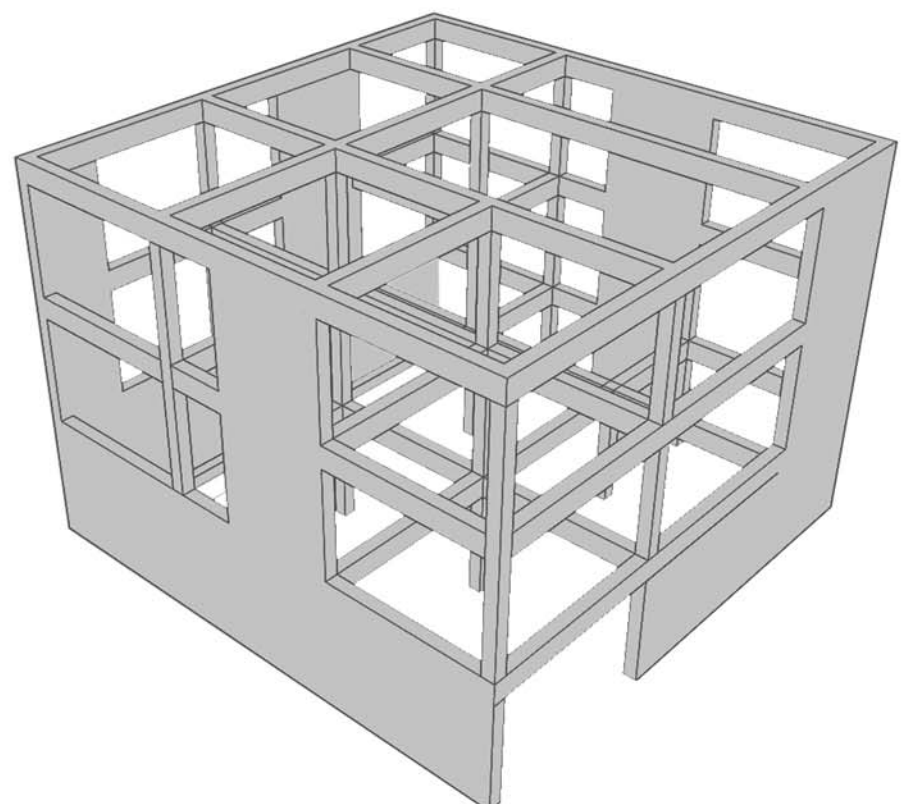
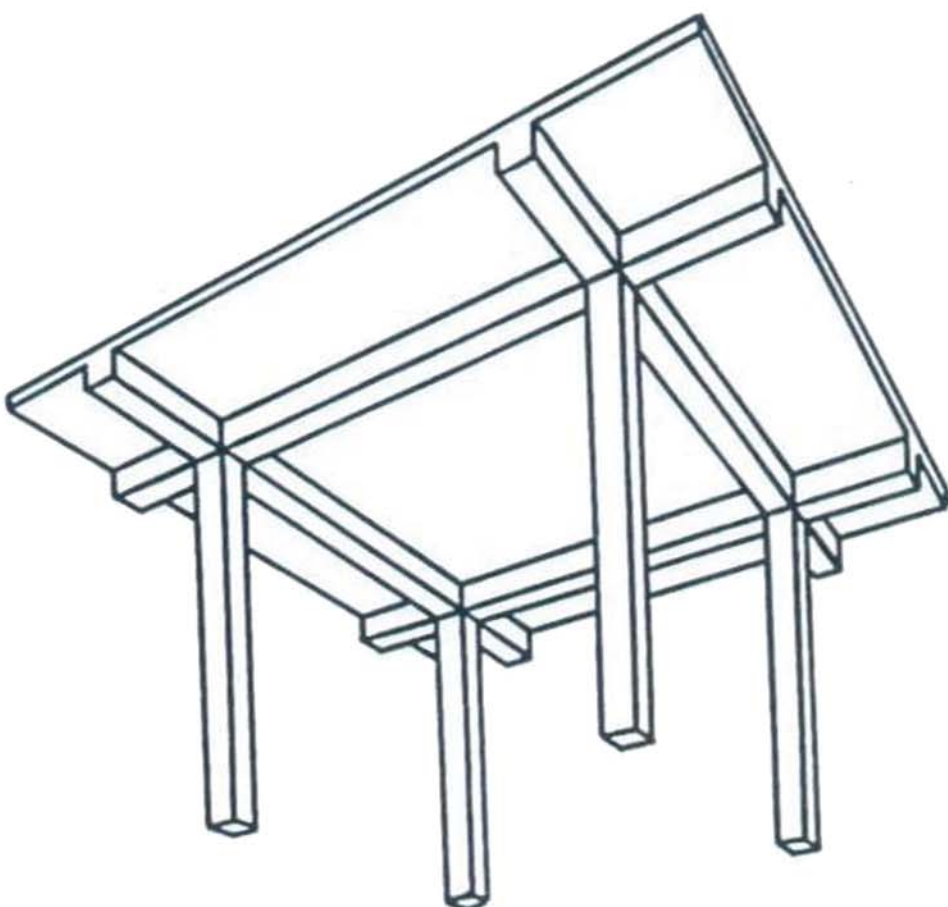
The ratio of shear wall in x direction is **1%**.

Area of shear wall on y direction:

$$0,3 \times 2,50 + 0,3 \times 2,25 + 0,3 \times 3 = 2,32 \text{ m}^2$$

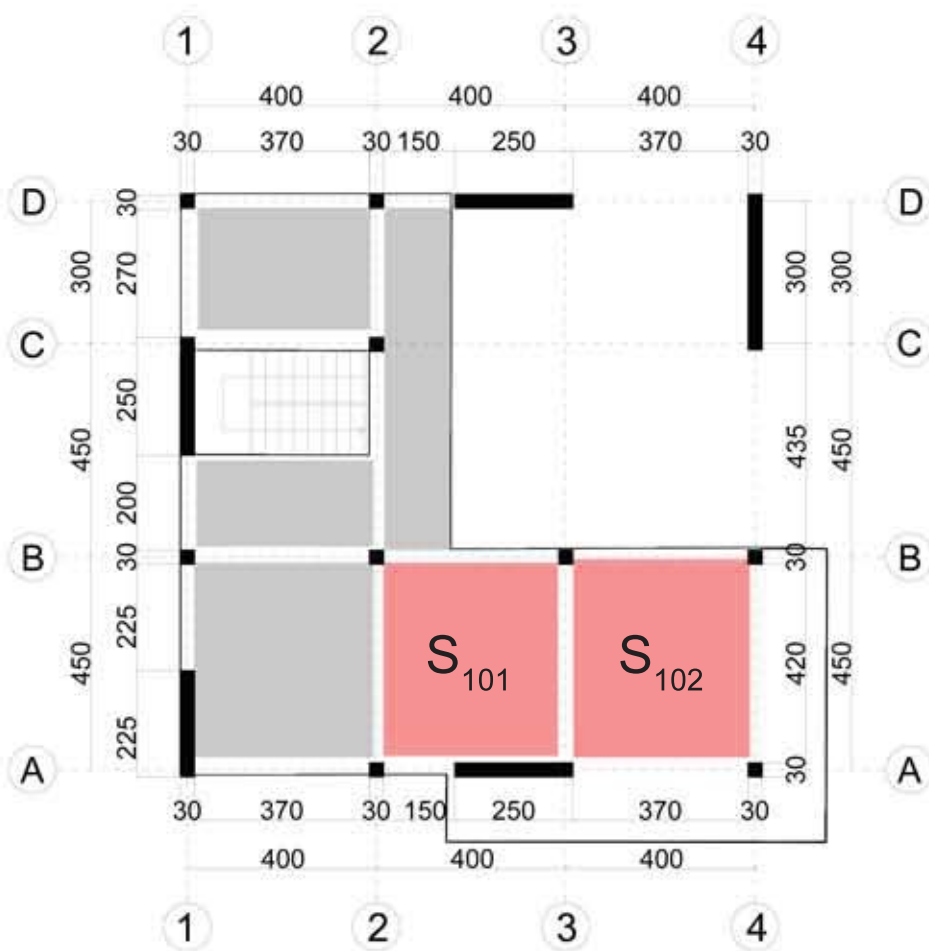
The ratio of shear wall in x direction is **1,6%**.

In x direction $0,75\% < 1\%$ and in y direction $0,75\% < 1,6\%$ so it is acceptable.



Two way solid slab system is chosen as slab system

FIRST FLOOR SLAB



$$\alpha = \frac{\text{length of continuous edges}}{\text{total length of all edges}}$$

$$\alpha_{S_{101}} = \frac{4+4,5+4,5}{4+4+4,5+4,5} = 0,76$$

$$\alpha_{S_{102}} = \frac{4+4,5+4,5}{4+4+4,5+4,5} = 0,76$$

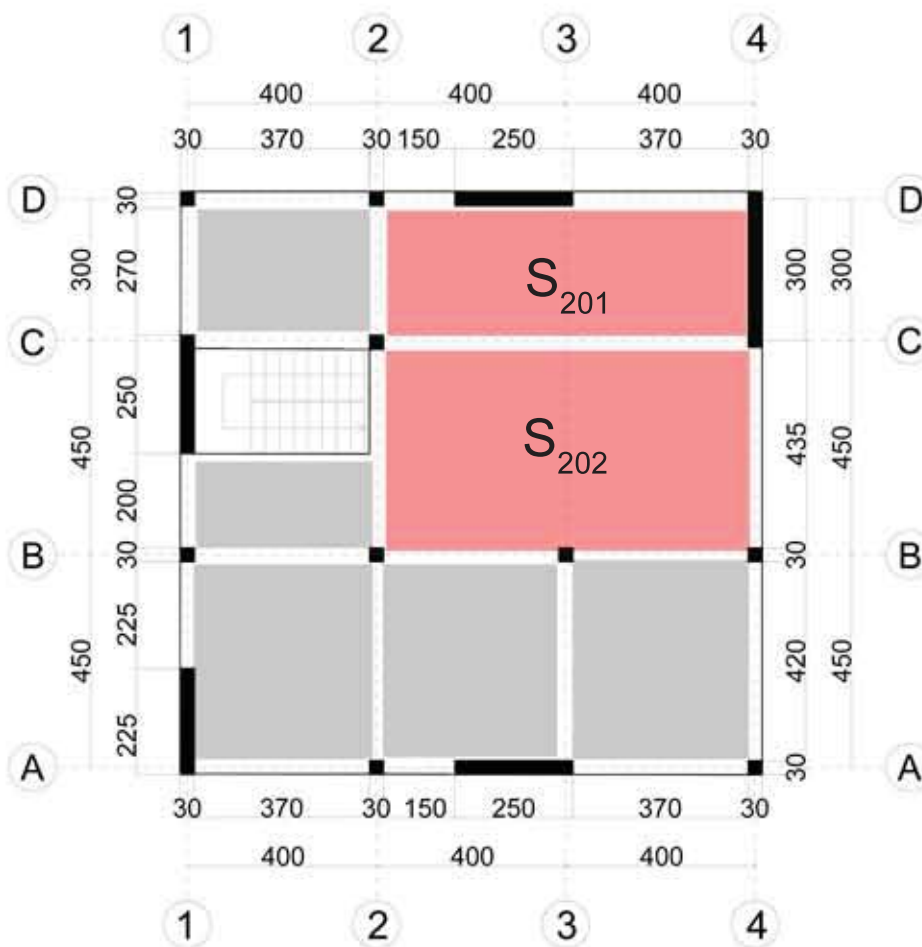
$$t \geq \frac{l_{\text{short}}}{15 + \frac{20}{\frac{l_{\text{long}}}{l_{\text{short}}}}} \times \left(1 - \frac{\alpha}{4}\right)$$

$$t_{S_{101}} = \frac{4}{15 + \frac{20}{\frac{4,5}{4}}} \times \left(1 - \frac{0,76}{4}\right) = 0,1 \text{ m}$$

$$t_{S_{102}} = \frac{4}{15 + \frac{20}{\frac{4,5}{4}}} \times \left(1 - \frac{0,76}{4}\right) = 0,1 \text{ m}$$

For first floor slab most critical slabs are s₁₀₁ and s₁₀₂.

ROOF SLAB



$$\alpha = \frac{\text{length of continuous edges}}{\text{total length of all edges}}$$

$$\alpha_{S_{201}} = \frac{3+3+8}{3+3+8+8} = 0,63$$

$$\alpha_{S_{202}} = \frac{8+8}{4,5+4,5+8+8} = 0,64$$

$$t \geq \frac{l_{\text{short}}}{15 + \frac{20}{\frac{l_{\text{long}}}{l_{\text{short}}}}} \times \left(1 - \frac{\alpha}{4}\right)$$

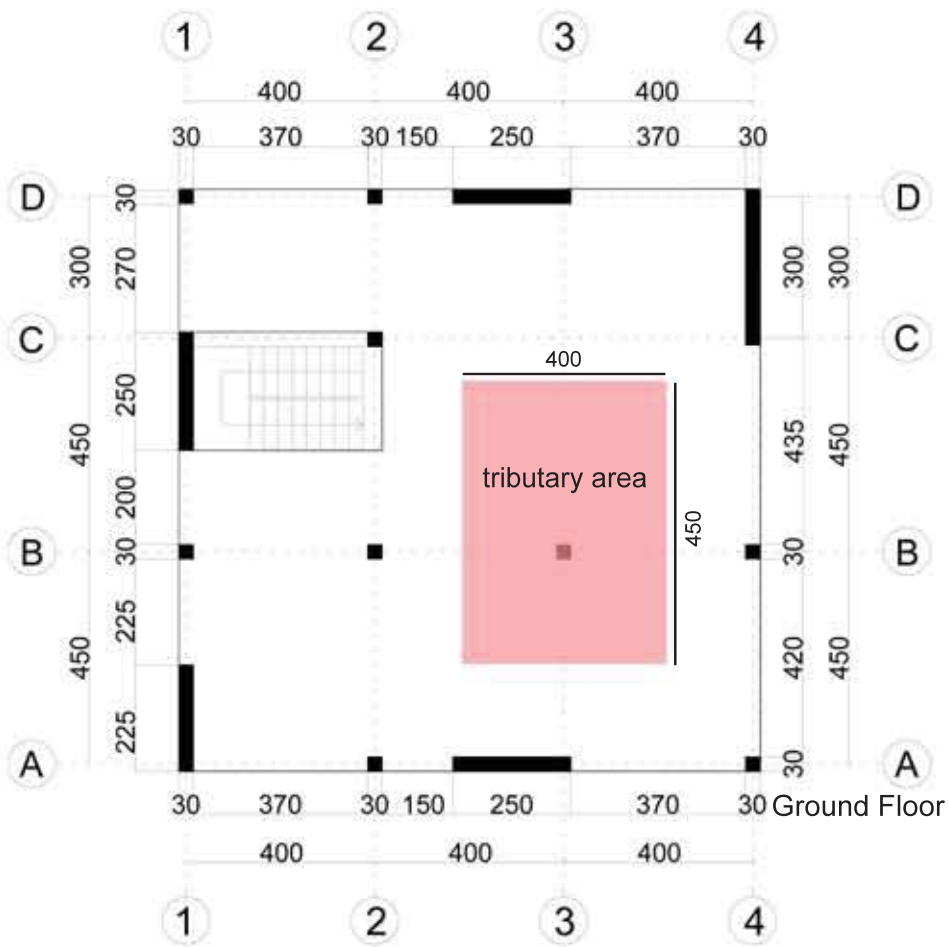
$$t_{S_{201}} = \frac{3}{15 + \frac{20}{\frac{8}{3}}} \times \left(1 - \frac{0,63}{4}\right) = 0,11 \text{ m}$$

$$t_{S_{202}} = \frac{4,5}{15 + \frac{20}{\frac{8}{4,5}}} \times \left(1 - \frac{0,64}{4}\right) = 0,14 \text{ m}$$

For first floor slab most critical slabs are s₂₀₁ and s₂₀₂.

Since the most critical slab is s₂₀₂ at roof slab and its thickness is 14 cm. Slob thickness should be more than critical slab thickness.

SLAB THICKNESS = 15 cm



$$\text{Beam depth} = \frac{\text{Longest span}}{12,5} = \frac{800}{12,5} = 64 \text{ cm}$$

Assume beam depth is 65 cm and wall height is 2,35m.

Design Loads for slab:

Dead Load:

own weight: $0,15 \times 2,4 = 0,36 \text{ t/m}^2$

levelling: $0,004 \times 2,4 = 0,096 \text{ t/m}^2$

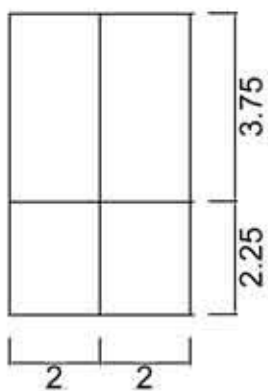
covering: $0,025 \times 2 = 0,05 \text{ t/m}^2$

plastering: $0,02 \times 2 = 0,04 \text{ t/m}^2$

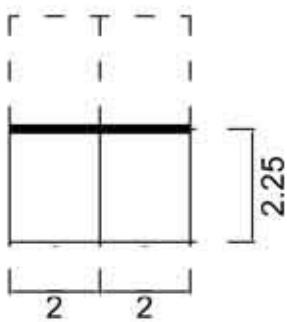
$$= 0,54 \text{ t/m}^2$$

Live Load = $0,2 \text{ t/m}^2$

$$\text{Total Load} = (1,4 \times 0,54) + (1,6 \times 0,2) = 1,08 \text{ t/m}^2$$



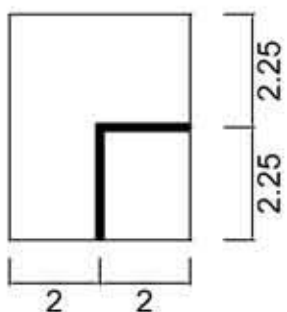
slab load: $(3,75 + 2,25) \times (2 + 2) \times 1,08 = 25,92 \text{ t}$



wall load: $(2,35) \times (2 + 2) \times 0,45 \times 1,4 = 5,92 \text{ t}$

slab load: $(2 + 2) \times (2,25) \times 1,08 = 9,72$

wall load: $(2,25 + 2) \times (2,35) \times 0,45 \times 1,4 = 6,29$



slab load: $(2 + 2) \times (2,25 + 2,25) \times 1,08 = 19,44$

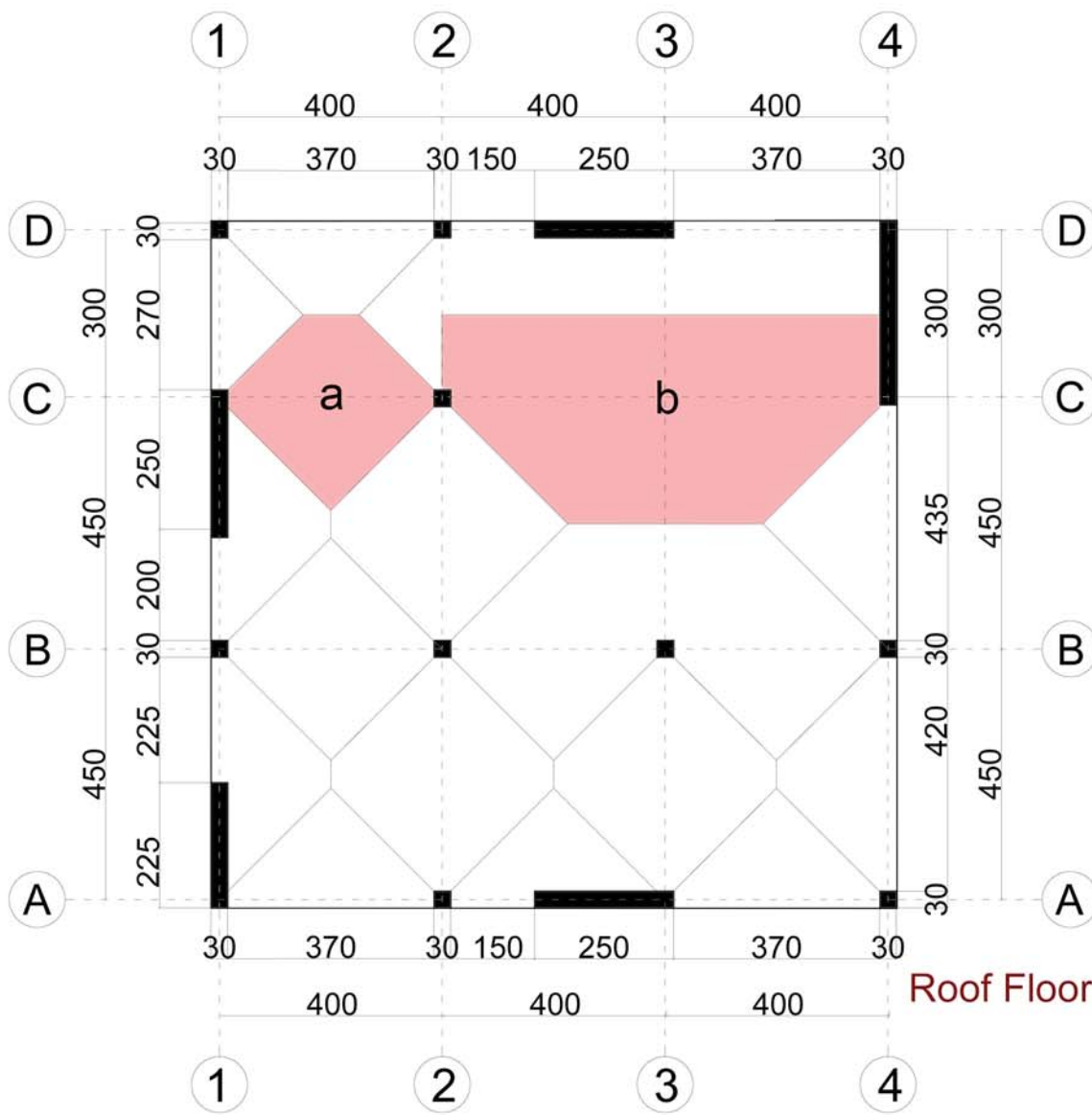
total load: $67,29 \text{ t} = 67290 \text{ kg}$

$$\text{Then; } A_c \geq \frac{N_d}{0,75 \times f_{cd}}$$

$$= \frac{67290}{0,75 \times 130} = 690,15 \text{ cm}^2$$

Since $\text{min } A_c < 750 \text{ cm}^2$

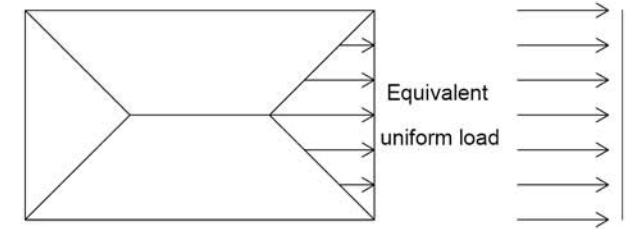
Column dimensions are 30cmx30cm according to TS-500



After calculating several beams, we realize that the most critical beam is roof beam which passes through the gallery

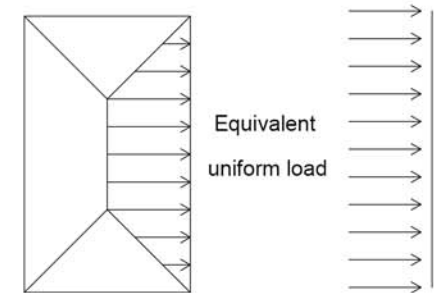
For Short Span

$$P_d \times \frac{l_{\text{short}}}{3}$$



For Long Span

$$P_d \times \frac{l_{\text{short}}}{3} \times \left(1,5 - \frac{0,5}{\left(\frac{l_{\text{long}}}{l_{\text{short}}}\right)^2} \right)$$



$$P_d = (1,4 \times DL) + (1,6 \times LL)$$

$$P_d = (1,4 \times 0,54) + (1,6 \times 0,2) = 1,08 \text{ t/m}^2$$

Load Region a :

$$w_{12} = 1,08 \times \frac{3}{3} \times \left(1,5 - \frac{0,5}{\left(\frac{4}{3}\right)^2} \right) + 1,08 \times \frac{4}{3} = 2,75 \text{ t/m}$$

Load Region b :

$$w_{24} = \frac{1,08 \times 8 \times 3}{2 \times 8} + 1,08 \times \frac{4,5}{3} \times \left(1,5 - \frac{0,5}{\left(\frac{8}{4,5}\right)^2} \right) = 3,79 \text{ t/m}$$

$$I_{\text{column}} = 1/12 \times 0,3 \times (0,3)^3 = 0,000675 \text{ m}^4$$

$$I_{\text{shear}} = 1/12 \times 2,5 \times (0,3)^3 = 0,0056 \text{ m}^4$$

$$I_{\text{shear2}} = 1/12 \times 3 \times (0,3)^3 = 0,0067 \text{ m}^4$$

$$I_{\text{beam}} = 1/12 \times 0,3 \times (0,65)^3 = 0,0068 \text{ m}^4$$

$$r_{12} = \frac{0,0068/4}{(0,0068/4) + (0,0056/3)} = 0,477$$

$$r_{21} = \frac{0,0068/4}{(0,0068/4) + (0,0068/8) + (0,000675/3)} = 0,612$$

$$r_{24} = \frac{0,0068/8}{(0,0068/4) + (0,0068/8) + (0,000675/3)} = 0,306$$

$$r_{42} = \frac{0,0068/8}{(0,0068/8) + (0,0067/3)} = 0,276$$

$$\text{FEM} = \frac{W \times l^2}{12}$$

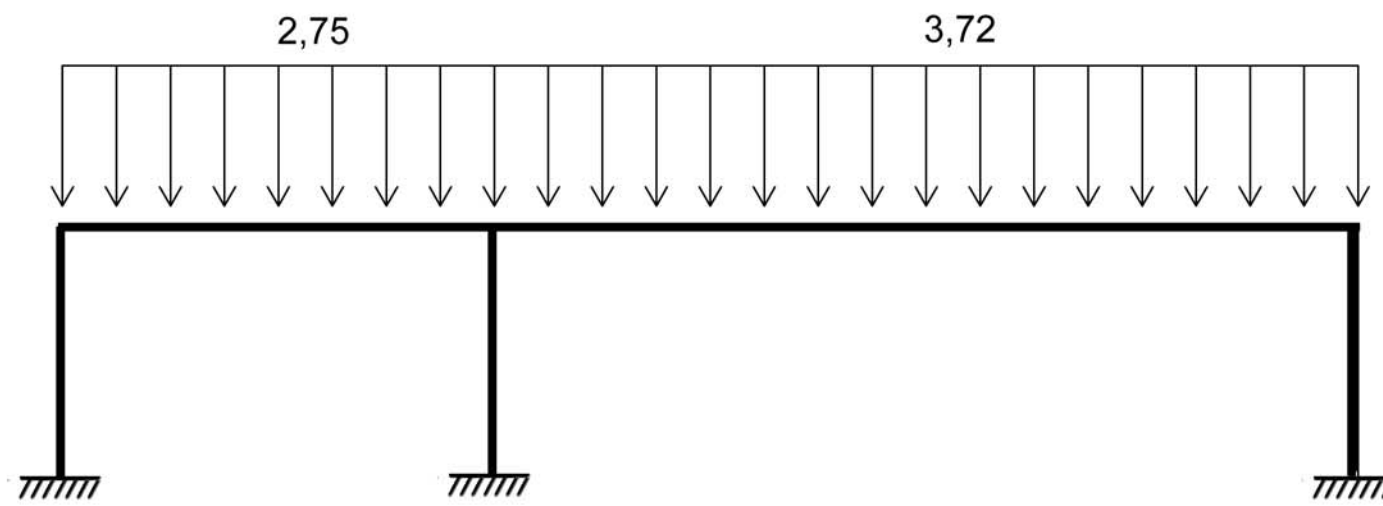
$$\text{mid-span moment} = \frac{W \times l^2}{24}$$

$$\text{FEM}_{1-2} = \frac{2,75 \times 4^2}{12} = 3,67 \text{ tm}$$

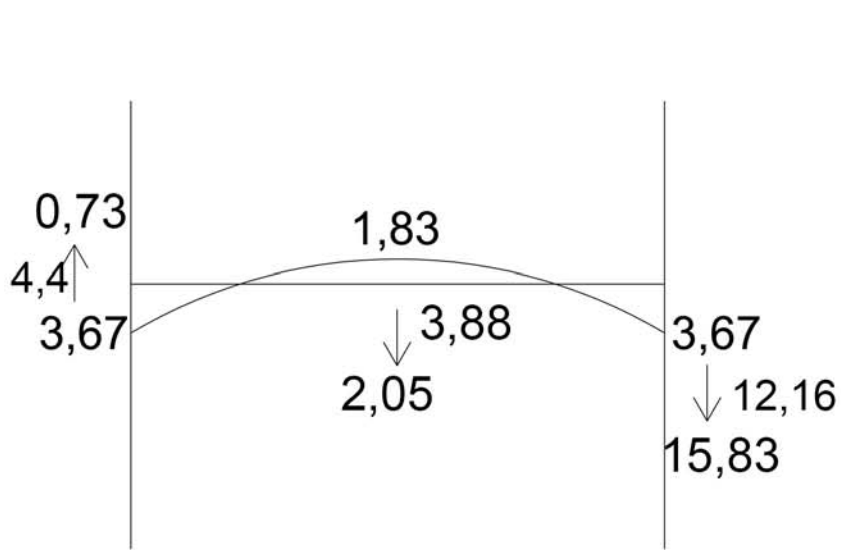
$$\text{mid-span moment}_{1-2} = \frac{2,75 \times 4^2}{24} = 1,83$$

$$\text{FEM}_{2-4} = \frac{3,79 \times 8^2}{12} = 20,21 \text{ tm}$$

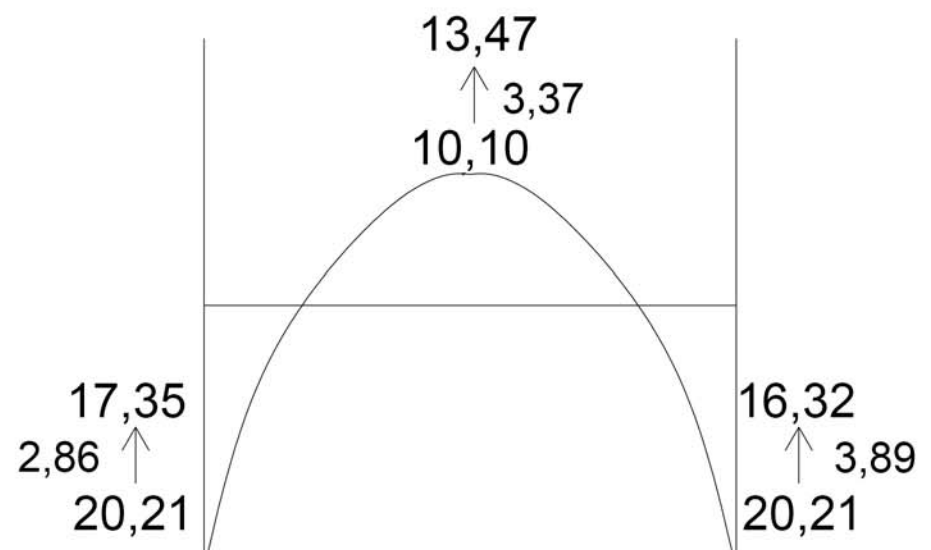
$$\text{mid-span moment}_{2-4} = \frac{3,79 \times 8^2}{24} = 10,10 \text{ tm}$$



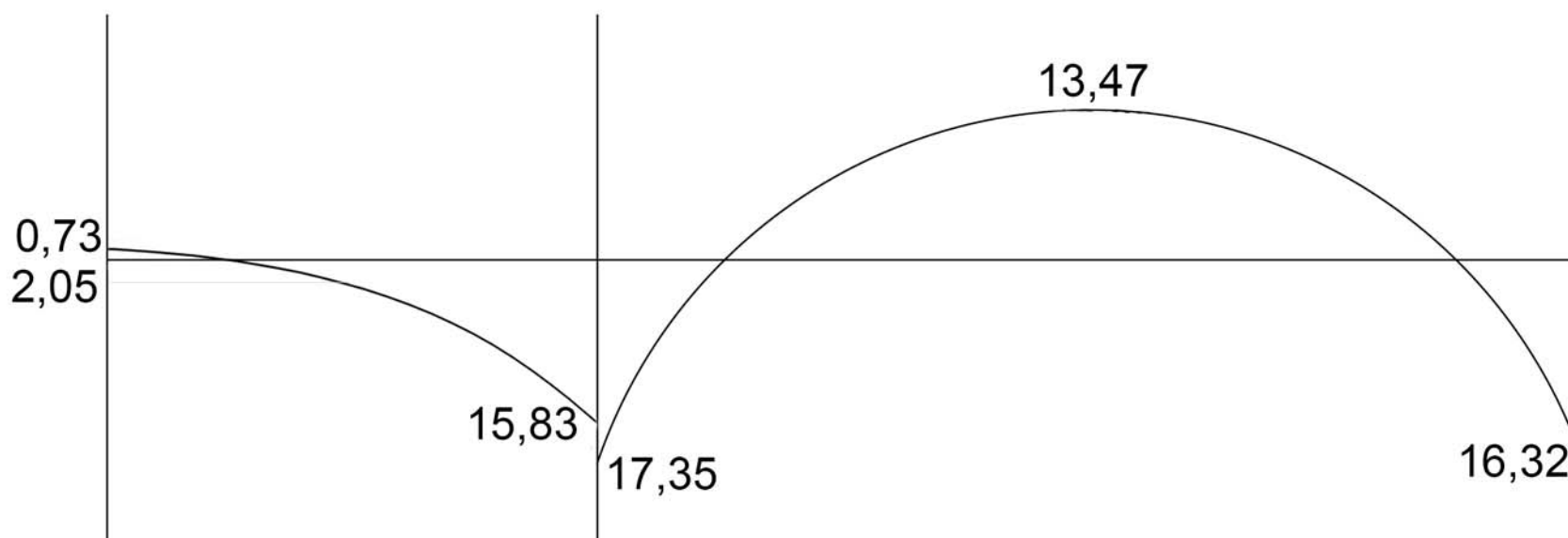
r	0,477	0,612	0,306	0,276
FEM	3,67	-3,67	20,21	-20,21
	-5,06	-0,87	2,78	-2,53
Σ_1	-1,39	-4,54	22,99	-22,54
	0,66	-11,29	-5,64	6,22
Σ_2	-0,73	-15,83	17,35	-16,32



Mid span: $\frac{12,16-4,4}{2} = 3,88$
 $1,83-3,88 = -0,34$



Mid span: $\frac{2,86+3,89}{2} = 3,37$
 $10,10+3,37 = 13,47$



$$K = \frac{bwxd^2}{M} \implies 0,025 = \frac{30xd^2}{1735000} \implies d = 38,02$$

$$h > d + 5 = 42,68 \quad \text{and} \quad h > 3t \quad \text{and} \quad t = 15 \implies h > 45$$

Beam Depth = 50 cm

